



## INTERNATIONAL JOURNAL FOR RESEARCH

IN APPLIED SCIENCE & ENGINEERING TECHNOLOGY

Volume: 7 Issue: IV Month of publication: April 2019

DOI: https://doi.org/10.22214/ijraset.2019.4658

www.ijraset.com

Call: © 08813907089 E-mail ID: ijraset@gmail.com



ISSN: 2321-9653; IC Value: 45.98; SJ Impact Factor: 7.177 Volume 7 Issue IV, Apr 2019- Available at www.ijraset.com

# Behaviour of Cold-Formed Z Purlin with and without Edge Stiffener (Lip) in Pre-Engineered Building

Stimiti S. Waghmare<sup>1</sup>, B.P. Nandurkar<sup>2</sup>, Dr. Ramesh Meghrajani<sup>3</sup>

<sup>1</sup>M. Tech. (Structure) Student, YCCE, Nagpur

<sup>2</sup>Professor of Civil Engineering, YCCE, Nagpur

<sup>3</sup> Neo Infrastructure consultants, Nagpur

Abstract: Cold-formed steel (CFS) sections are widely used in industrial and non-industrial construction due to its light weight, corrosion resistance, high strength to weigh ratio and economy. Zed and Channel section purlins are commonly used in PEB's. Cold-formed sections are more liable to buckling and flexure because of its minimum slenderness ratio. This paper introduces the buckling behaviour of cold-formed steel Zed purlins with edge stiffeners (lip) when subjected to wind load. Non-linear finite element method (ANSYS R15.0) is used to investigate the behaviour of Zed purlin with and without edge stiffener. Therefore result concluded that 30° inclination of edge stiffener (lips) is more desirable than other. But in actual practice edge stiffener of 45° inclinations is adopted.

Keywords: Cold-Formed Sections, Non-linear, Edge stiffener, ANSYS.

### I. INTRODUCTION

There are two main types of structural members in steel Construction one is hot-rolled and other is cold formed. The cold-formed section is made from steel sheet, strip, plate or flat bar in roll forming machines or by bending brake or by press brake operation. Cold formed steel is adopted over other material like timber and concrete because of:

- 1) High strength and stiffness
- 2) Erection and installation is fast and easy.
- 3) Lightness
- 4) Non-shrinking and Non-creeping in ambient temperature.
- 5) Low maintenance.
- 6) Economy in transportation and handling.

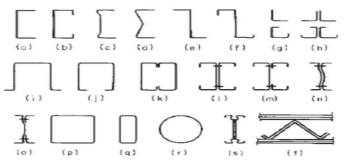


Fig.1 Various Cold-formed Section

The cold-formed steel design is time consuming as the cold-formed materials are susceptible to Local deformation under loads as the materials used are quite thin. Where as in hot-rolled members there is not much to worry about this failure as the materials are thicker than cold-formed. This local deformation comprises of two forms: local buckling and distortional buckling. When the stress reach the certain limit some part of the compression flange and the web buckles which is termed as local buckling. Although in distortional buckling the compression flange and the adjacent lip moves away from their original position resulting in weaking the section.



### International Journal for Research in Applied Science & Engineering Technology (IJRASET)

ISSN: 2321-9653; IC Value: 45.98; SJ Impact Factor: 7.177

Volume 7 Issue IV, Apr 2019- Available at www.ijraset.com

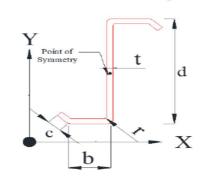
### A. Purlins

Purlins are the secondary framing of Pre-engineered building where it transfer load to the primary frame. Purlins are the beams provided to support the roof between the adjacent trusses. Angle sections, channel sections, Z sections are commonly used purlins. Cold formed purlins are made of high-strength steel. Cold formed Z and C are the workhorses of the industry. It can be designed as a continuous or simple-span member. Therefore it can be made continuous by overlapping and fastening. The Wind forces are acting normally on buildings and assume to act normal to the roof truss and the gravity loads passes through the centre of gravity of the purlin section. When the purlins are simply supported at the rigid frame rafters or trusses, the maximum bending moment will be  $\frac{\text{wl}^2}{8}$  about each axis where 'w' is the appropriate component of the load. Although, if sag rods are used it will provide lateral supports in Y-axis bending, requiring the purlins be treated as continuous beams and then maximum bending moment will be  $\frac{\text{wl}^2}{10}$ 

### II. METHODOLOGY

### A. Dimension of Z Purlin

Symbol	Z(mm)
d	200
b	60
С	20
t	1.5
r	4
L	20
	d b



The test specimen of Z sections are shown in Fig. 2 to Fig 5

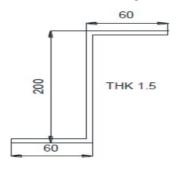


Fig. 2 Section properties of Z purlin without lip

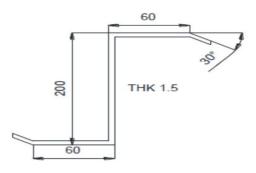


Fig. 3 Section properties of Z purlin with 30° lip

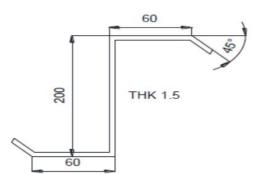


Fig. 4 Section properties of Z purlin with 45° lip

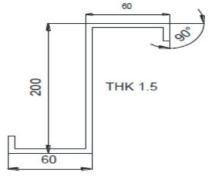


Fig. 5 Section properties of Z purlin with 90° lip

ISSN: 2321-9653; IC Value: 45.98; SJ Impact Factor: 7.177 Volume 7 Issue IV, Apr 2019- Available at www.ijraset.com

B. Material Properties of Cold-formed Steel

Table 1 Property of Cold-Formed Steel

COLD-FORMED STEEL NON-LINEAR		
DENSITY	$7850 \text{ kg/m}^3$	
ISOTROPIC ELASTICITY		
DERIVE FROM	Young's modulus	
YOUNGS MODULUS	2.00E+11 Pa	
POISSIONS RATIO	0.3	
BULK MODULUS	1.67E+11 Pa	
SHEAR MODULUS	7.69E+10 Pa	
BILINIEAR ISOTROPIC HARDENING		
YIELD STRENGTH	3.45E+08 Pa	
TANGENT MODULUS	1.45E+09 Pa	

Table 2 Specimens details

Specimen(mm)	Dimension(mm)	Length(mm)
Z-section without lips (Z1)	200x60x20x1.5	8000
Z-section with 30° lips (Z2)	200x60x20x1.5	8000
Z-section with 45° lips (Z3)	200x60x20x1.5	8000
Z-section with 90° lips (Z4)	200x60x20x1.5	8000

### III. FINITE ELEMENT MODELLING

The finite element methodology is a numerical analysis technique for getting approximate solutions to a wide selection of engineering issues. Most of the engineering issues these days create it necessary to get approximate numerical answer to issues instead of precise closed kind solutions. ANSYS will perform advanced engineering analyses quickly, safely and much by its sort of contact algorithms, time based mostly loading options and nonlinear material models. The basic plans behind the finite vary of parts having finite dimensions and reducing the structure having infinite degrees of freedom to finite degrees of freedom. Then the initial structure is that the assemblage of those components connected at a finite component analysis a complicated software system of ANSYS R.15 was used. The elastic properties of material were appointed to make a model cold formed steel Z section with and without lips.

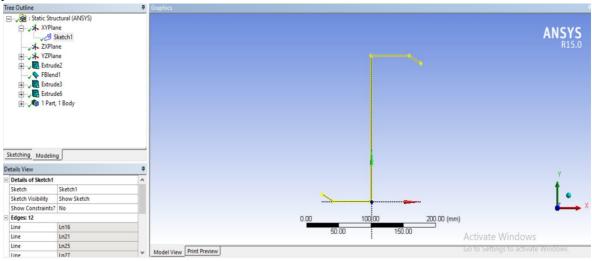


Figure 6 2-dimensiosnal Z purlin

ISSN: 2321-9653; IC Value: 45.98; SJ Impact Factor: 7.177 Volume 7 Issue IV, Apr 2019- Available at www.ijraset.com

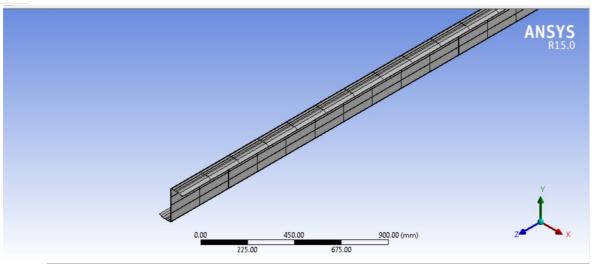


Figure 7 Typical Meshed model of Z purlin

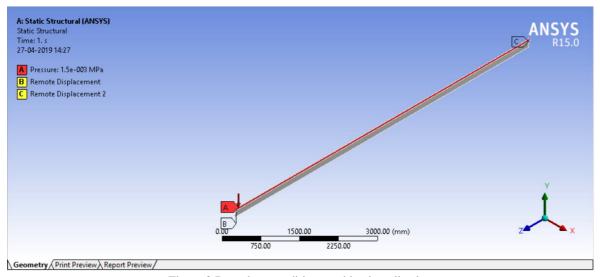


Figure 8 Boundary conditions and load application

### IV. RESULT AND DISCUSSIONS

The analysis of the purlin is done by using the software ANSYS R15.0.By applying boundary conditions and loads the results are calculated. The results are discussed in the form of deflection. The systematic results of Cold-formed Z purlin with lips and without lips are discussed and results were discussed.

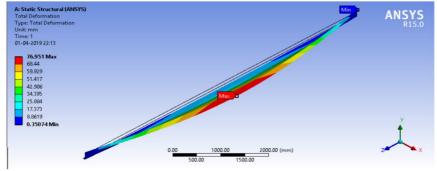


Figure 9 Deflection of Z purlin without lips

### International Journal for Research in Applied Science & Engineering Technology (IJRASET)

ISSN: 2321-9653; IC Value: 45.98; SJ Impact Factor: 7.177

Volume 7 Issue IV, Apr 2019- Available at www.ijraset.com

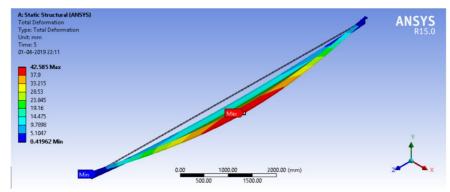


Figure 10 Deflection of Z purlin with 30° lips

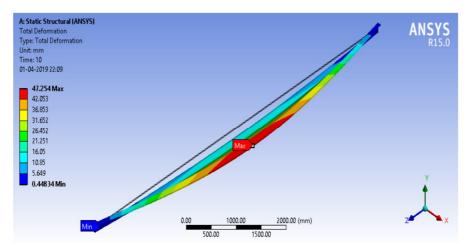


Figure 11 Deflection of Z purlin with 45° lips

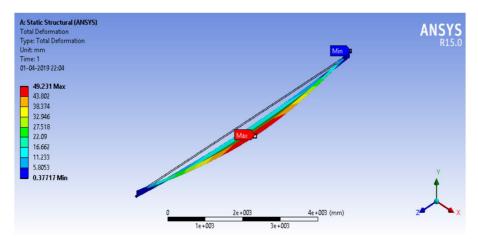


Figure 12 Deflection of Z purlin with 90° lips

**Table 3 Maximum Deformation** 

Specimen	Total Deformation (mm)
Z1	76.95
Z2	42.58
Z3	47.25
Z4	49.23



### International Journal for Research in Applied Science & Engineering Technology (IJRASET)

ISSN: 2321-9653; IC Value: 45.98; SJ Impact Factor: 7.177

Volume 7 Issue IV, Apr 2019- Available at www.ijraset.com

The deformation of the purlin with lips and without lips is shown in table no. 3. The maximum allowable limit of deformation is checked by referring IS Codes like IS800;2007, BS5950-5: 1998. The deformation of purlin without lips is 76.95mm which is maximum among all. The deformation of purlin with 30 lips is 42.58mm which is least among all.

### V. CONCLUSION

Comparative studies on Cold-formed Z purlin with lips and without lips are made and the following conclusions were made.

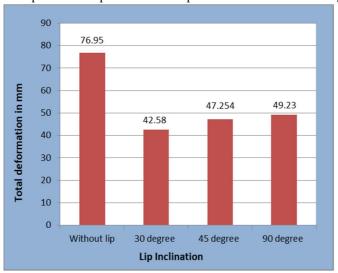


Figure 13 Total deformations vs. lip inclination

- A. The ultimate deformation of Cold-formed Z purlin without lips is maximum whereas the ultimate deformation of Cold-formed Z purlin with 30° lips is minimum.
- B. The Cold-formed Z purlin with 30° lips is more desirable due to its minimum deformation.
- C. Cold-formed Z purlin with 45° lips is adopted because the overlapping of the purlins can be done easily.
- D. The lips provided in the section to reduce the deflection of the member.

### REFERENCES

- [1] IS800:2007 (Indian Standard General Construction in Steel Code of practice)
- [2] IS801:1975 (Indian Standard Code of practice for use of Cold-Formed Light Gauge Steel Structural Members in General Building Construction)
- [3] SP:6(5)-1980 (Handbook for Structural Engineers 5. Cold-Formed, Light-Gauge Steel Structures.
- [4] IS875(Part3):2015 (Indian Standard Design Loads (Other than Earthquake) for Buildings and Structures Code of Practice
- [5] BS 5950-5:1998 (Structural use of steelwork in building Part 5 Code of practice for design of cold formed thin gauged sections)
- [6] AISI Manual 1998 (Cold-Formed Steel Design Manual)
- [7] Steel Designers Manual 2003
- [8] AISI/AISC 360-16 (Specification for Structural Steel Buildings)
- [9] A Newman "Metal Building Systems Design and Specification"
- [10] Dr. N Subramanian "Steel Structures Design and Practice"
- [11] Alomir H. Favero Neto, Luiz C.M. Vieira Jr., Maximiliano Malite "Strength and stiffness of cold-formed steel purlins with sleeved and overlapped bolted connections", Thin-Walled Structures, March 2016.
- [12] Lei Zhang, Gen-Shu Tong "Lateral bucking of simply supported C-and Z-section purlins with top flange horizontally restrained", Thin-Walled Structures, 2015.
- [13] W. Ye, C.J. Wang, D. J. Mynors, K. A. Kibble, T. Morgan, B. Cartwright, "Load-deflection behaviour of sleeved joints in modified Z purlin system", Thin-Walled Structures, 2013.
- [14] A. Biegus, "Causes of imminent failure damage and repair of Steel Building Purlins", ELSEVIER Archives of Civil and Mechanical Engineering, 2015.
- [15] Xiao-ting Chu, Jamie Rickard, Long-yuan Li, "Influence of lateral restraint on lateral torsional buckling of cold-formed steel purlins", ELSEVIER Thin-Walled Structures, 2005.
- [16] L Kemp, P E Dunaiski & W Bird, "Structural Behaviour of cold-formed profiles with emphasis on the Zeta-profile", ELSEVIER Journal of Construction and Steel Research, 1995.





10.22214/IJRASET



45.98



IMPACT FACTOR: 7.129



IMPACT FACTOR: 7.429



### INTERNATIONAL JOURNAL FOR RESEARCH

IN APPLIED SCIENCE & ENGINEERING TECHNOLOGY

Call: 08813907089 🕓 (24\*7 Support on Whatsapp)