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Dynamic Seismic Analysis of Multi Storey Buildings in Seismic Zone V

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Abstract: In India, multi-storied buildings are sometimes created because of high value and deficiency of land. Earthquake could be a phenomenon which might generate the foremost harmful forces on structures. Buildings ought to be created safe for lives by correct style and particularisation of structural members so as to possess a ductile sort of failure. To protect such civil structures from significant structural damage, the seismic response of these structures is analyzed along with wind force calculation and forces such as support reactions and joint displacement are calculated and included in the structural design for a vibration resistant structure. The primary objective is to make associate earthquake resistant structure by enterprise seismal study of the structure by static equivalent methodology of study and do the analysis and design of the building by using STAAD PRO software in both static and dynamic analysis

Keywords: Dynamic Seismic Analysis, Staad.Pro.

I. INTRODUCTION

Structural analysis is very important process for any structure as it ensures the stability, durability and economy of the structure. It is mainly concerned with the behavior of structure when it is subjected to external and internal forces. Frame structures (such as Concrete frame structures and Steel frame structures) entail various structural components such as stairs, ramp, beams, slabs, columns, foundation etc. vertical loads are the loads which act throughout the life span of buildings but horizontal loads may or may not act on the building. These loads are transferred from slabs to beams and then to columns. Then from columns, these loads are transferred to foundations and then ultimately to the soil. Soil must have enough bearing capacity to withstand the total load of the building without any deformations. Various tests are conducted on the soil in order to get the bearing capacity of the strata. Once the bearing capacity is known, then the type of foundation is decided. There are different types of foundations, shown in Figure 1. Which are being used according to the soil strata.

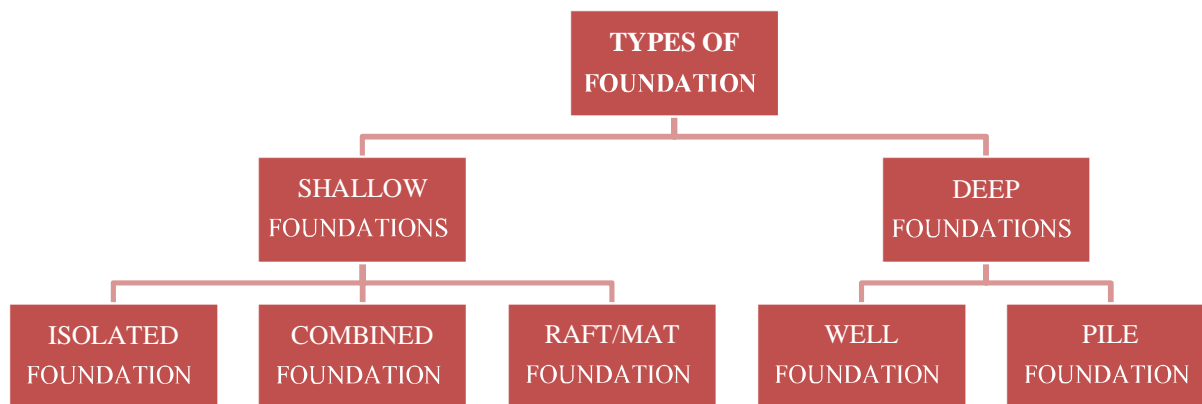


Figure 1. Types of Foundations.

Struts and columns, they both are basically the compression structural components which can resist compressive forces as well as lateral forces generating within the structure. Struts, unlike column, are an inclined member. Conventionally, struts were used in steel structures such as buildings, bridges etc but now they are being used in RCC frame structures also. This imparts large strength to the structure and helps in making it safe and economical. A strut can collapse due to buckling not by crushing. In simple words, a strut may be defined as a long, inclined colu

II. METHODOLOGY

A. Phase I: Modeling

For performing the current research work, total 6 models were prepared using Staad.Pro Software and they are mentioned as under:

Table 1. Description of Various Models

Type	Storey	Struts (bracing)
Type A	16 storey Building	None
Type B	16 storey Building	Single Cross Bracing
Type C	16 storey Building	Double Cross Bracing
Type D	20 storey Building	None
Type E	20 storey Building	Single Cross Bracing
Type F	20 storey Building	Double Cross Bracing

Other parameters of modeling are being represented in the table given below:

- Height of each storey = 3.3 m
- No. of bays in X-direction = 8
- No. of bays in Z-direction = 6
- Panel of each bay = 5.5 m x 6.5 m

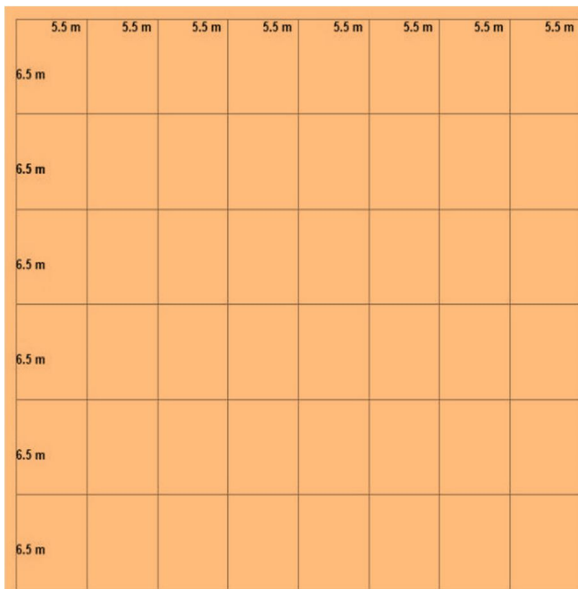


Figure 2. Plan of Building.

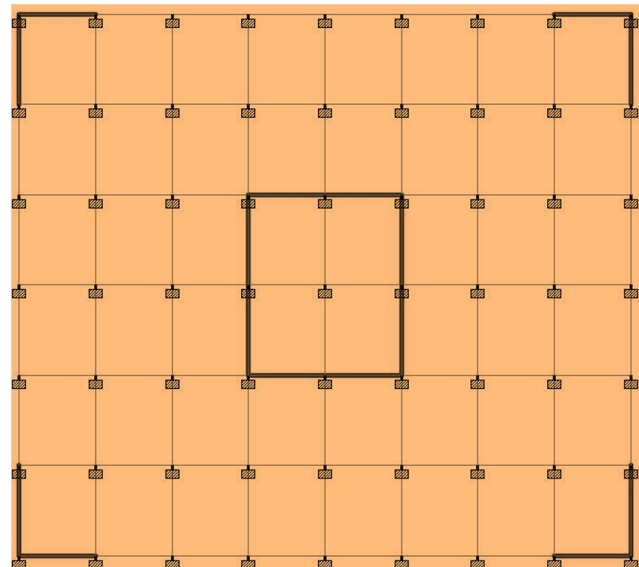


Figure 3. Location of Struts (Single Cross-Strut) in Plan.

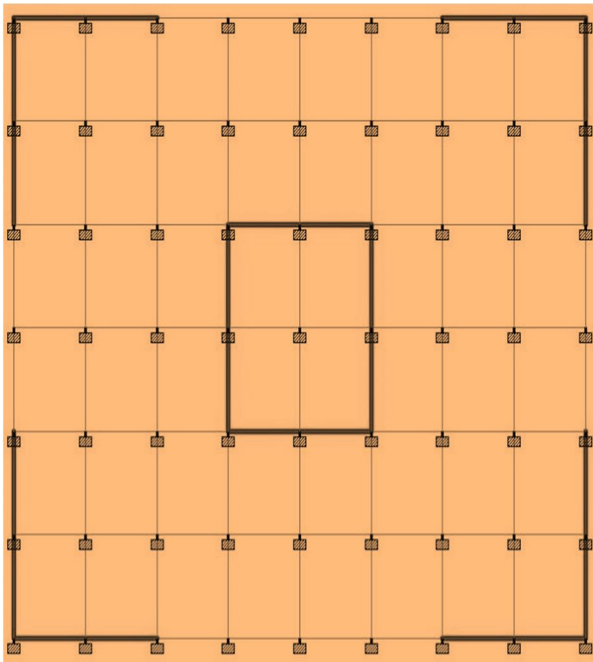


Figure 4. Location of Struts (Double Cross-Strut) in Plan.

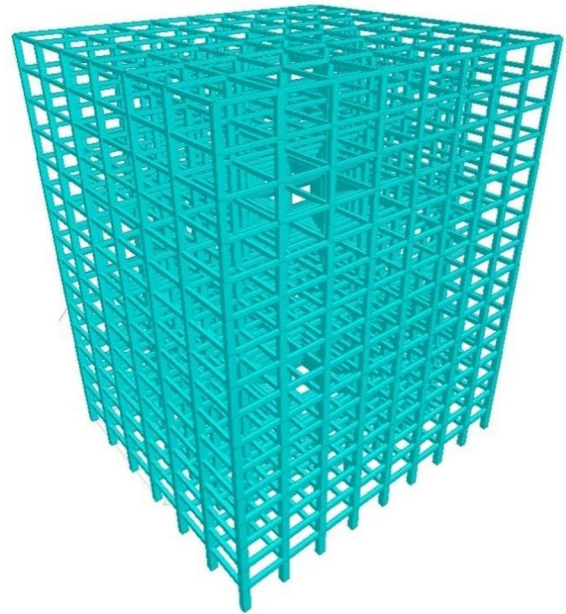


Figure 5. Rendered View of Type A Building.

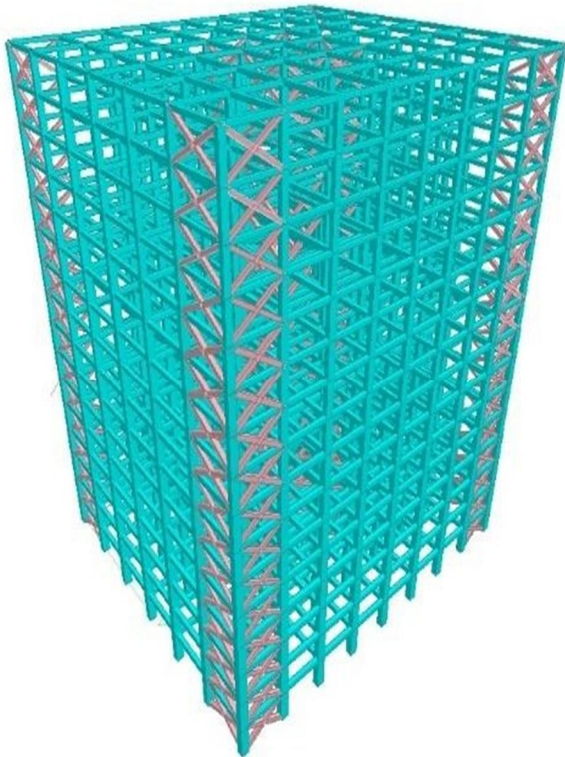


Figure 6. Rendered View of Type B Building.

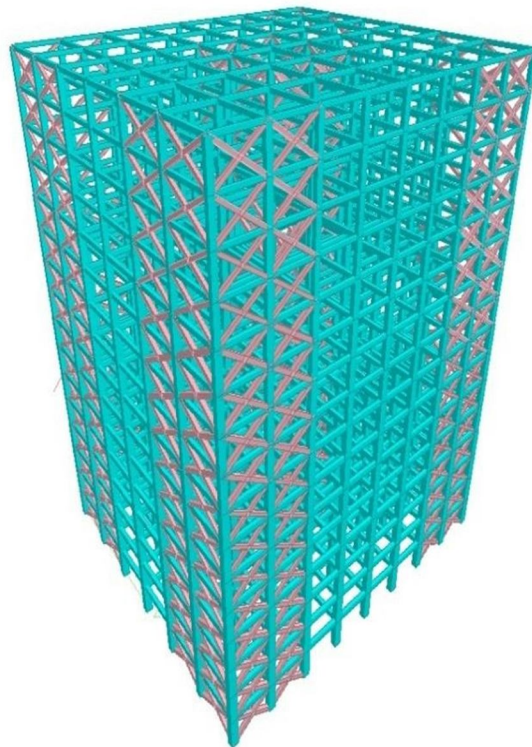


Figure 7. Rendered View of Type C Building.

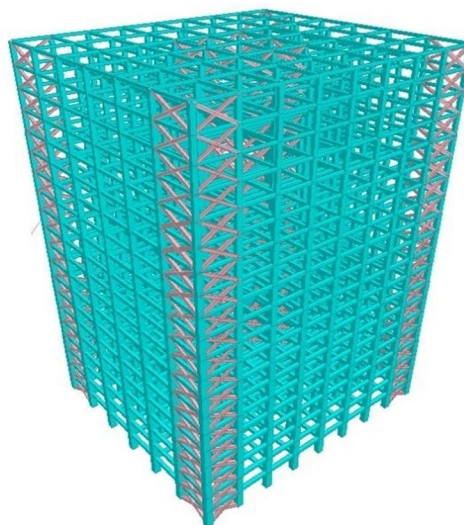
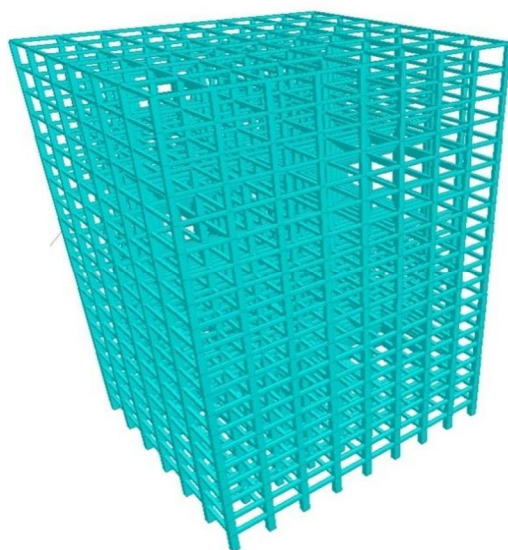


Figure 8. Rendered View of Type D Building. Figure 9. Rendered View of Type E Building.

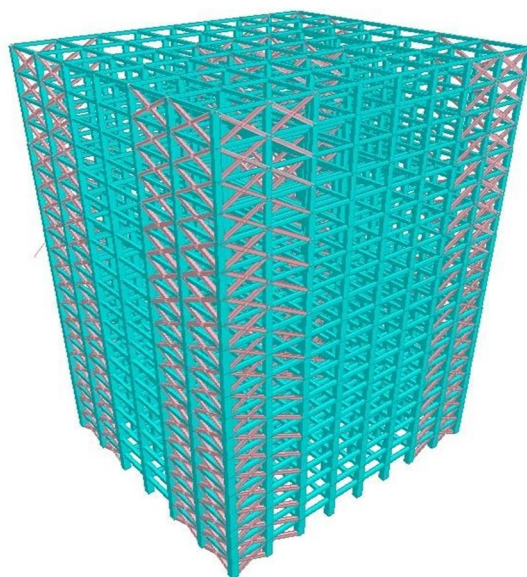


Figure 10. Rendered View of Type F Building.

Material properties which were assigned to different storey buildings are mentioned in table 2.

Table 2. Material Properties of Columns and Beams

Type of building	Floors	Column size (mm)	Beam size (mm)	Strut Size (mm)
Type A, B & C	1 st to 4 th floor	900 x 750 mm	600 x 450 mm	450 x 450 x 12 mm
	5 th to 8 th floor	825 x 650 mm	575 x 380 mm	350 x 350 x 12 mm
	9 th to 12 th floor	750 x 525 mm	450 x 380 mm	450 x 450 x 8 mm
	13 th to 16 th floor	600 x 380 mm	380 x 300 mm	350 x 350 x 8 mm
Type D, E & F	1 st to 4 th floor	980 x 875 mm	600 x 500 mm	500 x 500 x 12 mm
	5 th to 8 th floor	900 x 750 mm	600 x 450 mm	450 x 450 x 12 mm
	9 th to 12 th floor	825 x 600 mm	575 x 380 mm	350 x 350 x 12 mm
	13 th to 16 th floor	750 x 525 mm	450 x 380 mm	450 x 450 x 8 mm
	17 th to 20 th floor	600 x 450 mm	380 x 300 mm	350 x 350 x 8 mm

B. Phase II: Seismic Analysis

Total 6 models (3 for 16 storey building and other 3 for 20 storey building) were prepared in Staad.Pro software using new seismic code IS: 1893-2016 with dynamic seismic analysis. Relative seismic parameters were considered such as seismic zone, type of structure, importance factor etc. Seismic zone V was taken for the present study. After the analysis, all the results were recovered from various sources of software.

III. RESULTS AND DISCUSSION

The so obtained results have been collected from the staad.pro are now represented in various forms such as tables and figures. With the help of such representations, the results were compared between the same storey building having different bracings. Displacement, being the main factor which was used to evaluate the structure's lateral stability, was recorded at three different locations (displacement of corner columns, edge columns and central column) as shown in the figure below.

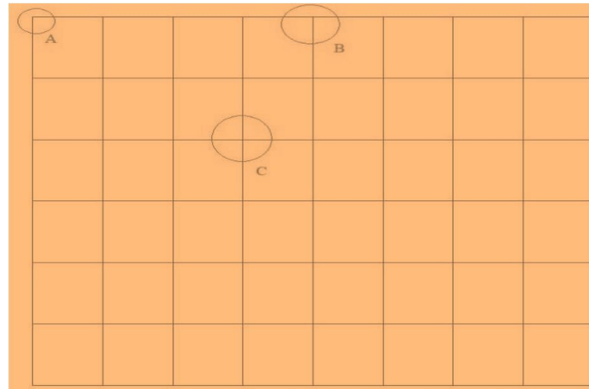


Figure 11. Location of Columns for Displacement Results.

Results of 16 Storey Building and 20 Storey Building.

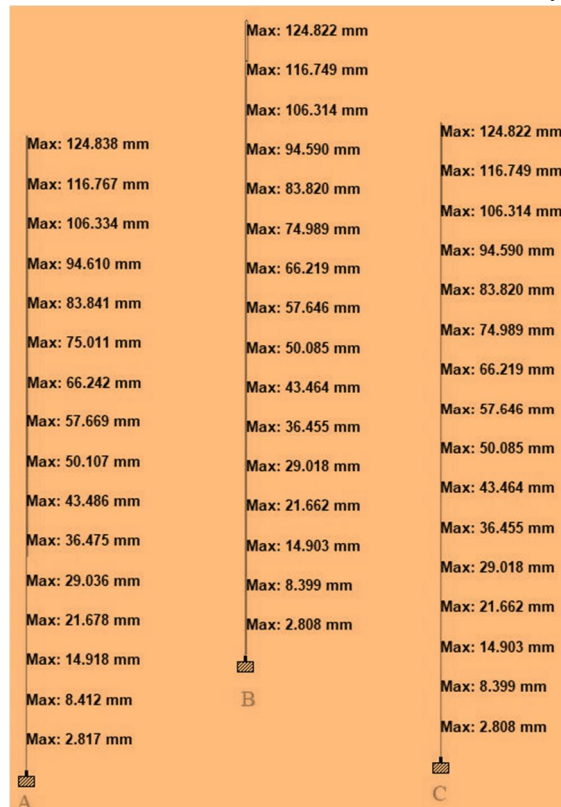


Figure 12. Displacement of Type A Building.

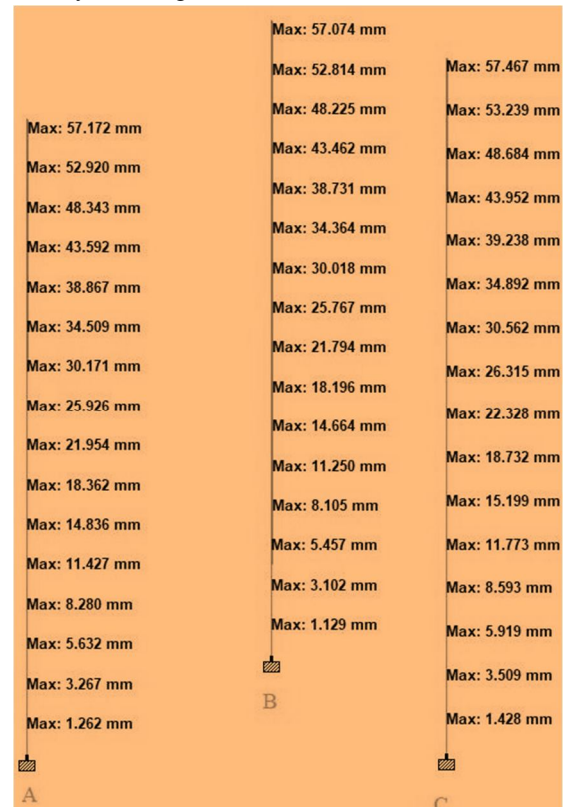


Figure 13. Displacement of Type B Building.



Figure 14. Displacement of Type C Building.

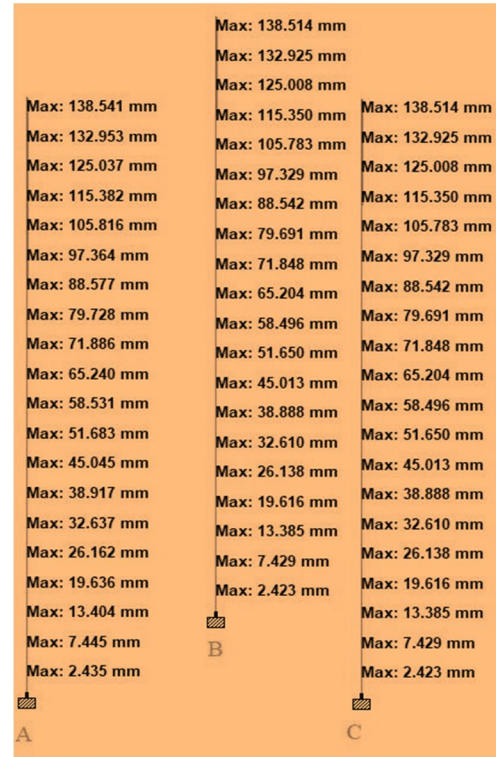


Figure 15. Displacement of Type D Building.



Figure 16. Displacement of Type E Building.

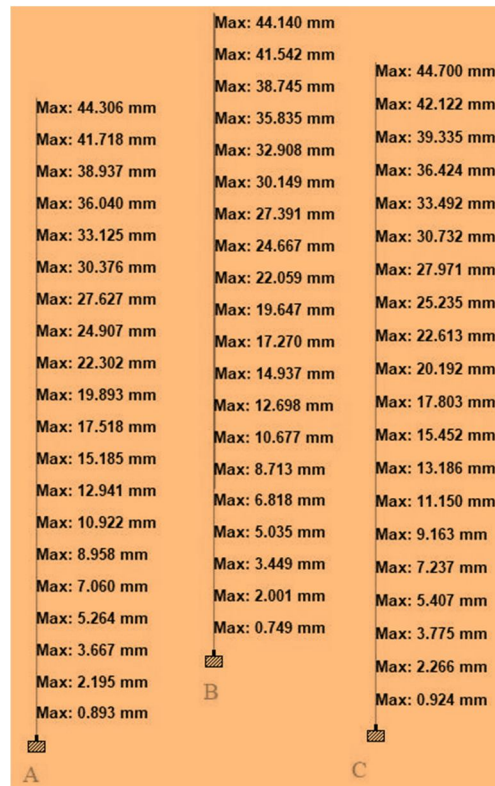


Figure 17. Displacement of Type F Building.

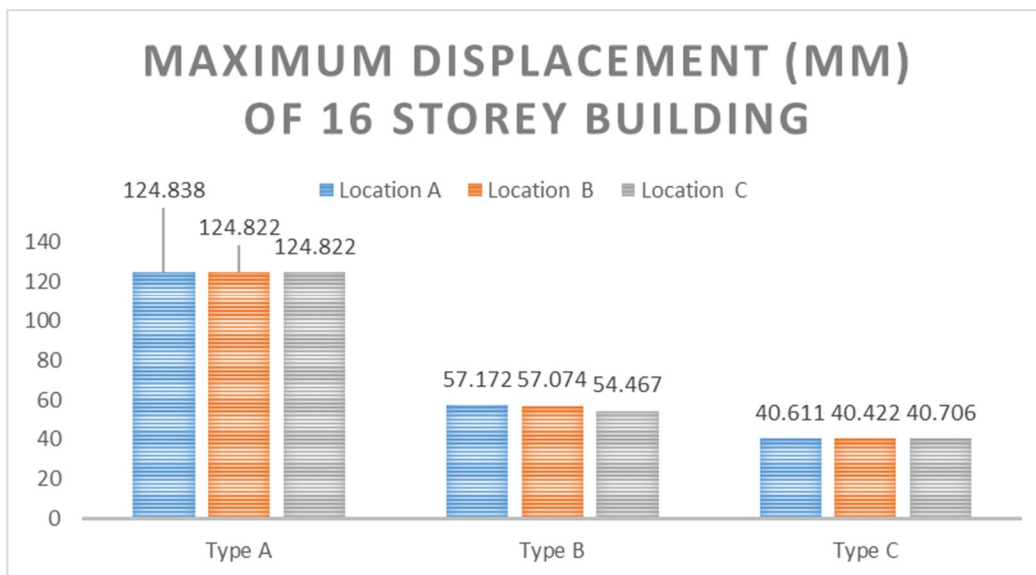


Figure 18. Maximum Displacement of 16 Storey Building.

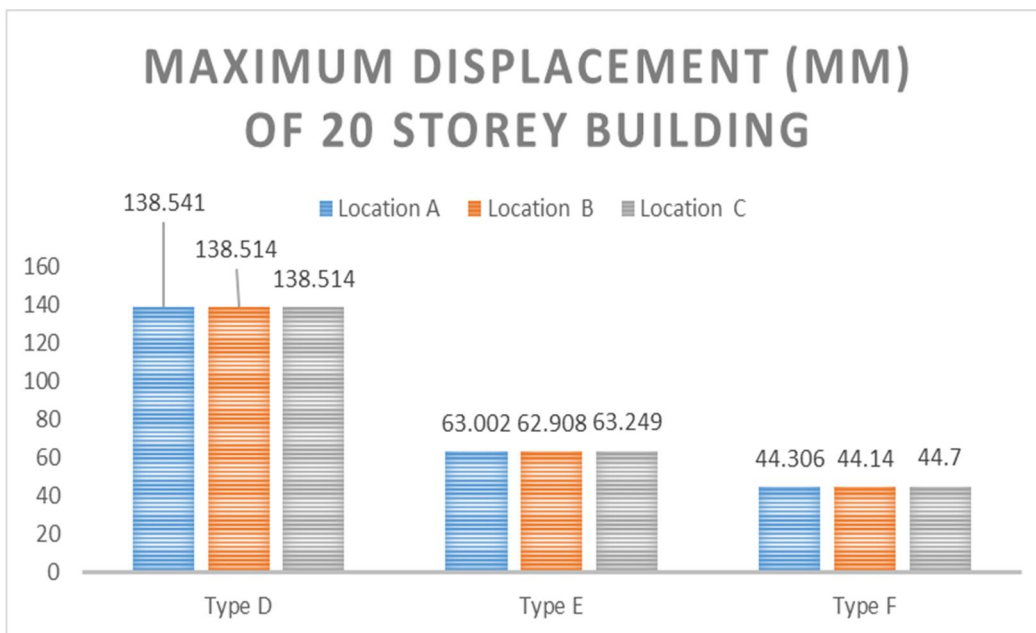


Figure 19. Maximum Displacement of 20 Storey Building.

Table 3. Material Quantity of 16 Storey Building.

	Concrete (m3)	Reinforcement Steel Kn	Steel Section(Kn)
Type A	3580.8	3764.36	-
Type B	3580.8	2461.04	4199.82
Type C	3580.8	2293.34	6299.73

Table 4. Material Quantity of 20 Storey Building.

	Concrete (m3)	Reinforcement Steel (Kn)	Steel Section (Kn)
Type D	5046.8	5151.55	-
Type E	5046.8	3736.36	5777.98
Type F	5046.8	3174.66	8339.71

IV. CONCLUSION

The present investigational study evaluates the high-rise building with and without struts as well as central core, the obtained results were studied carefully. From the represented results, following are the conclusions drawn for this current study:

- 1) For 16 storey building, the maximum displacement of column in Type A building, Type B and Type C building is 124.838 mm, 57.152 mm and 40.702 mm respectively. Therefore, the ratio of displacement of type B to type A is 0.46 and the ratio of displacement of type C to type A is 0.33.
- 2) For 20 storey building, the maximum displacement of column in Type D building, Type E and Type F building is 138.514 mm, 63.250 mm and 44.70 mm respectively. Therefore, the ratio of displacement of type E to type D is 0.46 and the ratio of displacement of type F to type D is 0.32.

Therefore, from the above observations it can be finally concluded that there was a reduction of 54 % in maximum displacement when single cross strut bracing was provided in 16 Storey Building and 20 Storey Building. But with the addition of double cross strut bracing, there was a huge reduction of 67 % in maximum displacement of 16 Storey and 20 Storey Building. Therefore, evaluation of both the buildings reflects that the utilization of cross type strut can be done effectively.

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