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Examining Seismic Response for Rigid Building Frame Having Composite Column

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Abstract: Concrete filled steel tubes are generally used in Beams, Columns, Piers and caissons for deep foundations. The steel tube functions as the formwork for casting the concrete and hence, construction cost is reduced. The prime focus of the present work is to study the behavior of RCC structure under the effect of seismic loads provided with composite columns. This research study comprises of seismic analysis with the design of rigid frame with Reinforced Concrete column, Concrete encased steel and steel tube encasing concrete columns. These cases are designed based on IS 1893:2016 using ETABS software. The result shows that steel encased concrete is performing better than conventional column. The construction practices is the only difficulty arises in such of composite column.

Keywords: Composite, Concrete encased steel, CFST, RCC

I. INTRODUCTION

Concrete-filled tubes are a dynamic sectional typology that consists of a circular or square-shaped steel tube filled with concrete on the inside as shown in figure below. The combined effect of a ductile material (steel) and a cohesive material (concrete) provides the section with very interesting mechanical properties. Since the 1970s, concrete-filled steel tubes have been used in industrial facilities in northern China. In workshop or industrial buildings, the column typically resists axial load and bending. For nearly 50 years, concrete-filled steel tubular columns have been used in China. They've been used in a variety of buildings, bridges, and other structures, including a number of industrial facilities (Han & Li, 2014). CFST members' high resistance, stiffness, and beneficial dynamic behavior met the requirements of heavily loaded industrial facilities. A four-story (G+3) RC building structure in Warangal, Telangana State, India, investigated by (Oggu et al., 2020) has been chosen as a case study. In addition, the structure will be extended vertically by two stories above its current configuration. Visual inspection and nondestructive tests are used to update the structural and material inputs to the analytical model, which is then followed by an analytical study. Self-Compacting Concrete Behavior Filled steel tube (CFST) fabrication located at U.S. Federal Courthouse, Seattle, WA, USA is done by (Sumalatha, 2020). The ultimate load capacity of composite circular hollow steel tubes with different grades of Self Compacting Concrete infill is tested. Steel tubes of various lengths, thicknesses, and grades are compared. The obtained results were compared to those of the American Concrete Institute (ACI), Euro Code-4 (EC-4) and modelling was done using the FL (Fuzzy Logic) technique, which is a soft tool in Matlab-R2018b. Guwahati, in north-east India, is in seismic zone V and is located in the Assam gap of the Himalaya. The study also supplements India's earthquake-resistant building codes and guidelines for new building construction (Ghione et al., 2021; Sarmah & Das, 2018). They were tailored and graded in accordance with (FEMA365, 2000) guidelines, which are more comprehensive when dealing with other building structures. The findings will be useful in developing local policy to arrange the building stock for appropriate remedial measures. The current practices of selection and scaling methods available in international codes such as ASCE 7-05, ASCE 7-10, and (ASCE, 2016) are presented and evaluated in this paper. A case study for a far field site in the town of Alipur in the Delhi region is presented. This paper shows how to scale ground motion using spectral acceleration as the intensity measure and amplitude scaling and spectral matching methods on a two-story three-dimensional RCC frame structure (Mulchandani et al., 2018).

II. METHODOLOGY

In this research work, case study of building considered originally located in Warangal, Telangana, India is carried for modelling in which RCC, Concrete Encased Steel and Steel Encased Concrete Column is been analyzed carried by Seismic Zone-III using ETABS software. Building responses of the structure are computed and then compared with the other cases. Loads considered are taken in accordance with the IS-875 (Part1 & Part2), IS-1893:2002/2016 & load combinations are according to IS-875(Part5).

A. Structural Details

The following below is the detail of Case Study to be analyzed in the investigation-

Table 1 Proposed Model Cases for the Research Study

Description	G+3	G+8
Building with Conventional RCC Column	RC1	RC3
Building with 4.2 m height Conventional RCC Column at ground storey	RC2	RC4
Building with Concrete Encased Steel Column	CF1	CF3
Building with 4.2 m height Concrete Encased Steel Column at ground Storey	CF2	CF4
Building with Steel Encased Concrete Column	SE1	SE3
Building with 4.2 m height Steel Encased Concrete Column at ground Storey	SE2	SE4

Note: The cases are categorized in two groups in which first group is of (G+3) building frame and second group is of (G+8) building frame.

The data of structure used in this thesis is in the form of tabulation considered for design and analysis of frame are given below-

Table 2 Structural Specification for the study

PARTICULARS	STRUCTURAL PROPERTIES
Reference Model (Case Study) Location	Warangal, Telangana, India
Total Built-Up Area	57.5 X 17.5 m
Number of Stories	G+3, G+8, G + 13
Nominal Floor to floor Height	3.2 m
Size of Columns	230X 530 mm, 300 X 610 mm
Beam Size	230 X 430 mm, 230 X 480 mm, 280 X 640 mm
Slab thickness	100 mm
Dead load	IS 875 Part-1
Live load	IS 875 Part-2
Roof live load	IS 875 Part-2
Earthquake load	(IS 1893 (Part 1), 2016)

B. Description of Model Cases of the Study

The grid lines drawn is non-uniform throughout the length and width of the building. The building shown below is asymmetry model taken first by Praveen Oggu et.al (2020) in which there are 18 bays having varying center to center distance of 2.4 m , 3 m, 3.5m , 3.7m along Y-axis and there are 5 bays having varying center to center distance of 3.5m , 4.2 m , 5.9m, along X-axis. The total length along X-axis is about 17.5 m. The length along Y-axis is 57.5 m. This cases signifies that RCC frame building with uniform storey height having conventional RCC slab having thickness 100 mm.

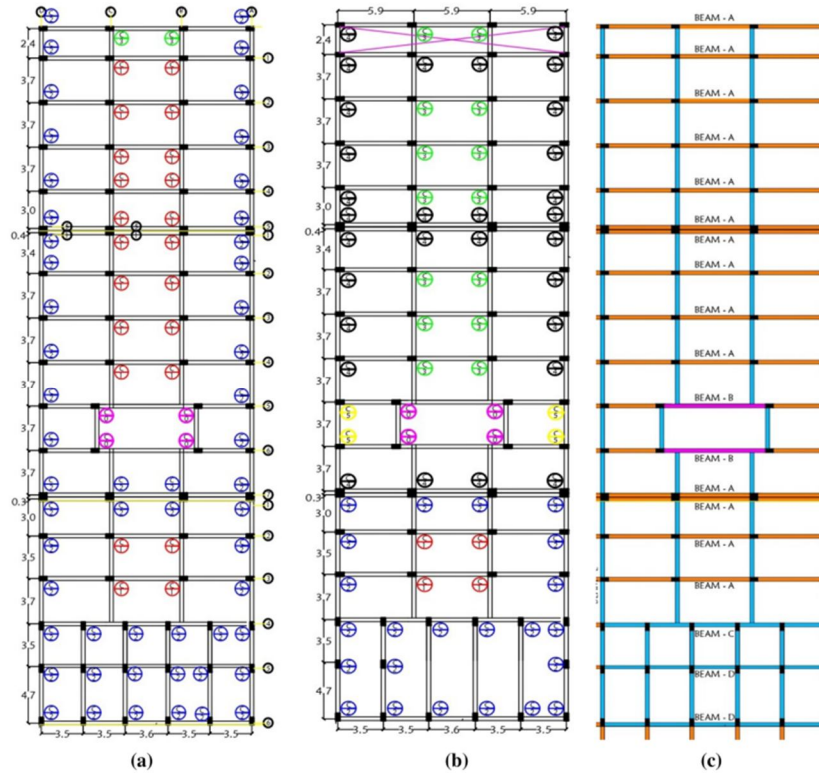
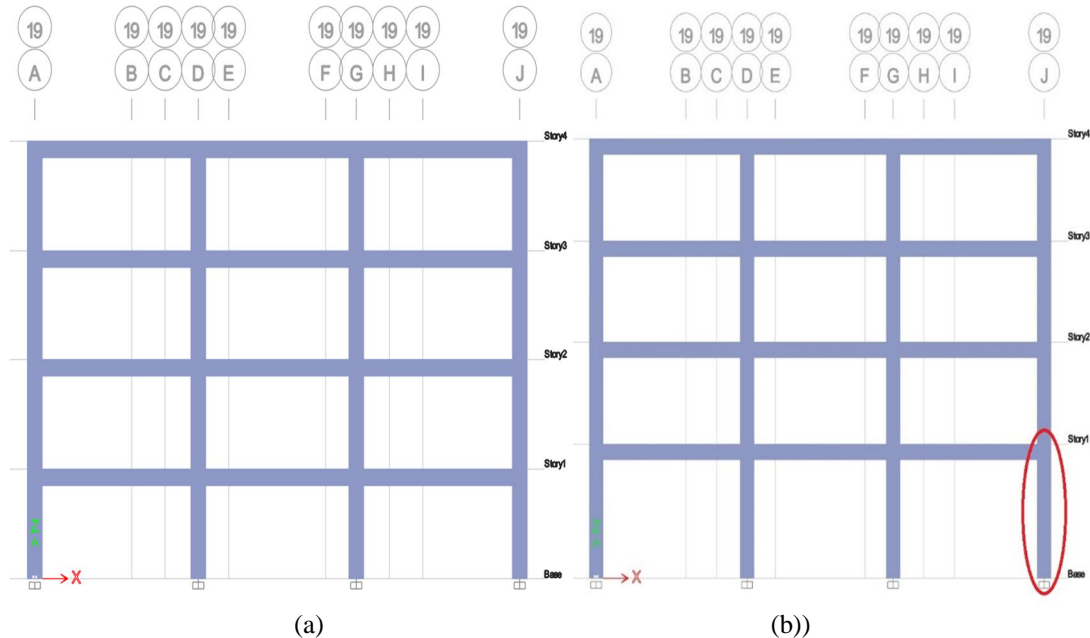


Fig. 3.3 Building Floor plan (a) Location of columns from the ground level (b) Location of Columns from G+2 level (c) Location of Beams (Source: Praveen Ogu et.al (2020))

1) *Model Based on Conventional RCC Column:* The columns applied in models are made with conventional RCC material with Case RC1, RC3 having nominal height of storey about 3.2 m for both (G+3 and (G+8) building frames. Later, the model RC2 and RC4 is made with column height 4.2 m at the ground storey only and the rest having nominal height 3.2 m for both (G+3 and (G+8) building frames.



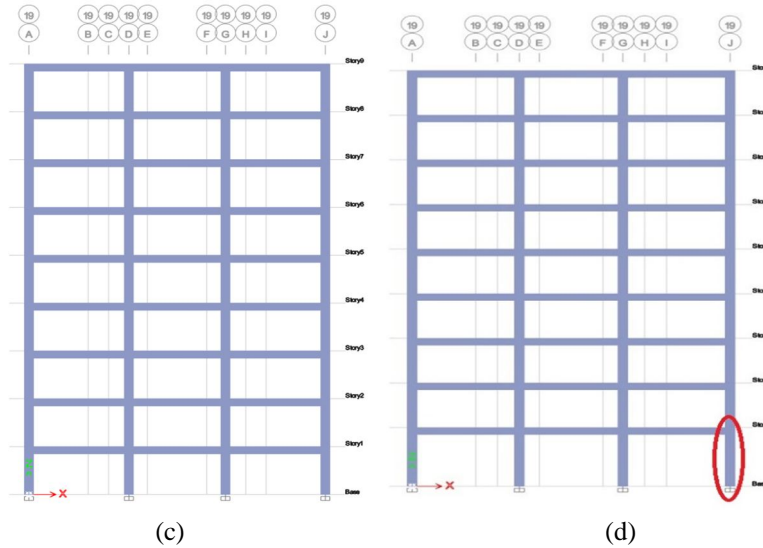


Fig. 3.4 Elevation View of (a) RC1(b) RC2 (c) RC3 (d) RC4

2) *Based on Concrete Encased Steel Column:* The building having (G+3), (G+8) storey are categorized as the building frame with CF1, CF3 respectively have composite column (concrete encased steel) with nominal height of storey about 3.2 m. Similarly, the model having concrete encased columns with 4.2 m height column at the ground storey and the rest storey have 3.2 m column height represents as CF2 and CF4 model.

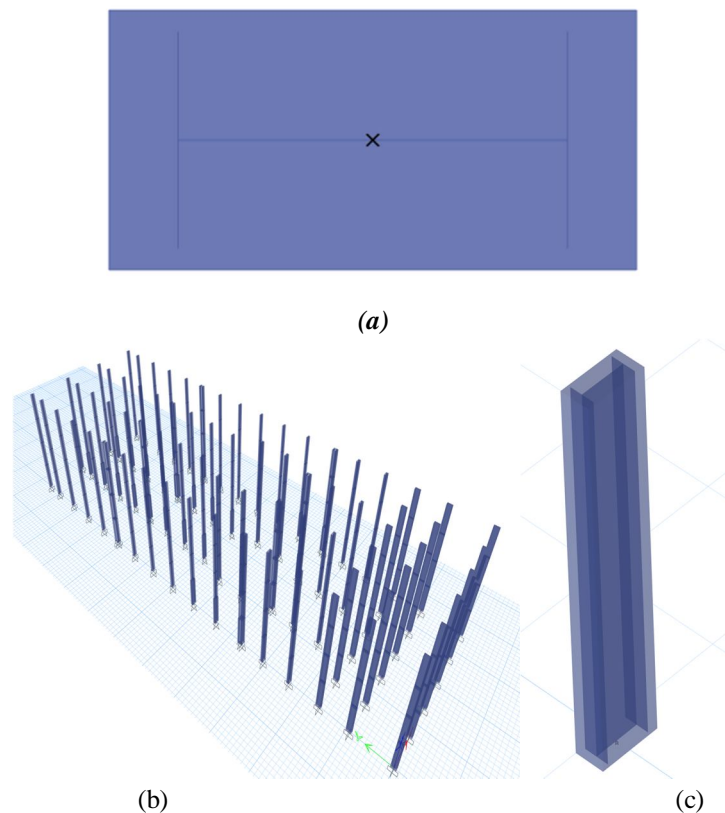


Fig. 3.5 Concrete Encased Column incorporated in the Considered Models (a) Concrete Encased Steel Columns Provided in Building (b) 3-D view of CES Columns (c) Plan View of CES Column

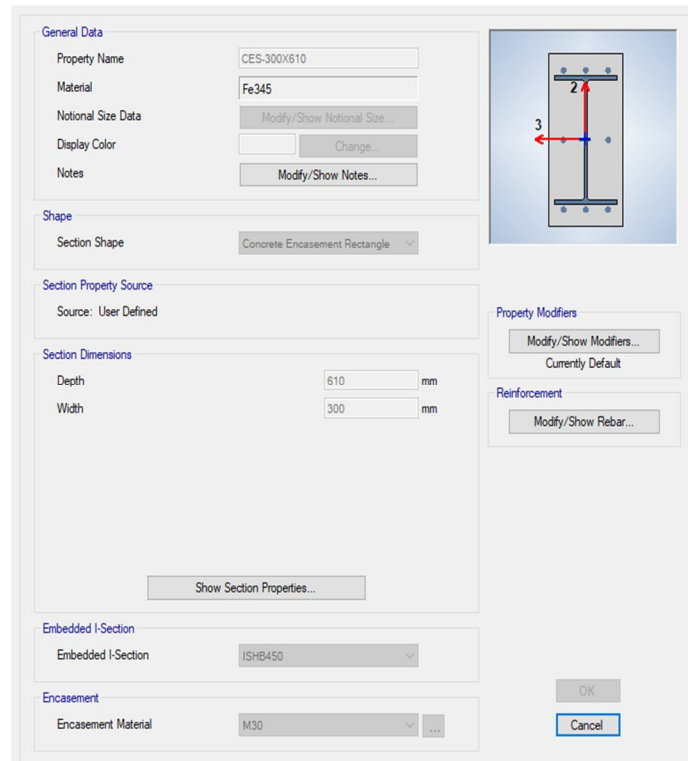


Fig. 3.6 Concrete Encased Steel Column Assigned for Modelling

- 3) *Model Based on Steel Encased Concrete Column:* The building having (G+3), (G+8) storey are categorized as the building frame with SE1, SE3 respectively have composite column (steel encased concrete) with nominal height of storey about 3.2 m. Similarly, the model having concrete encased columns with 4.2 m height column at the ground storey and the rest storey have 3.2 m column height represents as SE2, SE4.

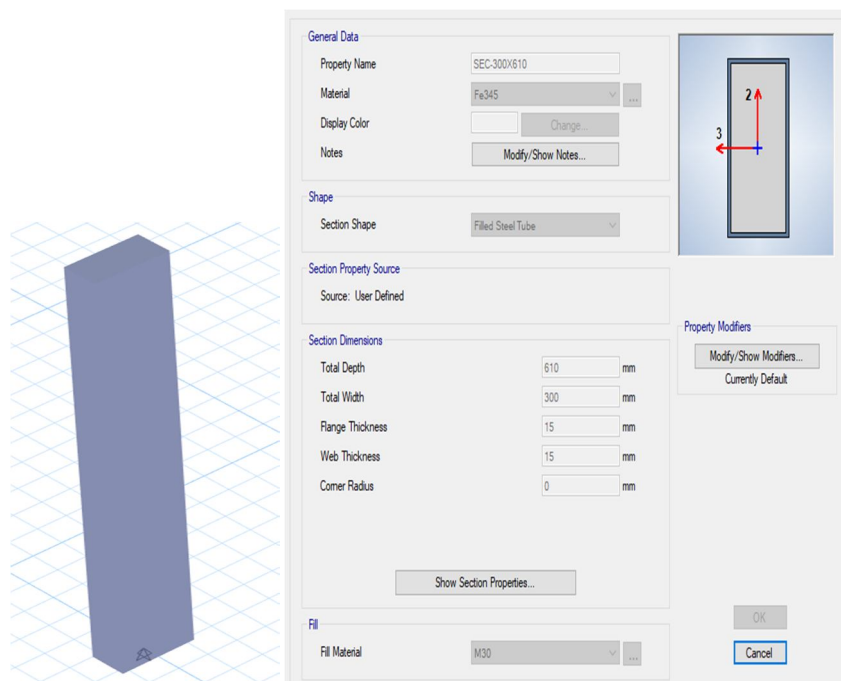


Fig. 3.7 Steel Encased Concrete Column Assigned for Modelling

III. LOADING SPECIFICATION & CALCULATIONS COMMON FOR ALL FRAMES USED IN SOFTWARE

The loads which are to be applied in the project is discussed under following clauses below in which their calculation detail is also been discussed such as Primary load, Seismic Load & their load combination etc.

A. Primary Loads Applied for Analysis

In Software, the loads are taken in the form of load cases i.e., primary load cases and the load combination of primary load cases also which are used same for all frame buildings. Firstly, here are the primary load cases which have been used in ETABS software analysis are given below in table 3.4 with their load type & numbers-

Table 3 Primary Load Cases

Load Case Number	Load Type	Name
1	Dead Load	DL
2	Live Load	LL
3	Seismic Dynamic Load	RSAX
4	Seismic Dynamic Load	RSAY

Load Calculations Used for All Frame Cases

The calculated load acting on the structures of dead load, floor live load, roof live load is given below-

1) Dead Load (D.L)

In this analysis, dead load includes dead load of the slab, dead load of beam & column, dead load of external walls and dead of internal walls. DEAD LOAD is designated as D.L in ETABS. Self-Weight of Slab/Plate = (unit weight of concrete X thickness of slab) = $25 \times 0.10 = 2.5 \text{ KN/m}^2$

The floor finish load is considered as dead load and applied in the slab. The value of floor finish is 1 KN/m^2 . The roof treatment load considered as dead load and assigned in the slab. The value of Roof treatment is 1.5 KN/m^2 . The Outerwall is taken 230 mm thick and inside wall is taken as 150 mm thick. The wall load is considered as dead load.

Self-Weight of Outer Wall = (unit weight of concrete X dimension of wall) = $20 \times 0.23 \times 3.2 = 14.72 \text{ KN/m}$

Self-Weight of Inner Wall = (unit weight of concrete X dimension of wall) = $20 \times 0.15 \times 3.2 = 9.6 \text{ KN/m}$

2) Live Load (L.L)

In this research, live load includes live load for all the floors as it is considered from the commercial building category given in IS 875 Part -2 and live load for roof is also considered from same above code. LIVE LOAD is designated as L.L. Here Live load for all the floors considered for the study is 3 KN/m^2 .

IV. RESULTS AND DISCUSSION

A. Displacement Report

Table 4 Displacement value for (G+3) building frame

Storey	Displacement for RC1 (mm)	Displacement for RC2 (mm)	Displacement for CF1 (mm)	Displacement for CF2 (mm)	Displacement for SE1 (mm)	Displacement for SE2 (mm)
Storey 4	17.546	18.527	16.092	17.766	10.837	12.921
Storey 3	14.305	15.764	12.975	14.972	8.667	10.794
Storey 2	9.454	11.607	8.465	10.845	5.587	7.696
Storey 1	4.076	6.438	3.384	5.788	2.19	3.982

Table 5 Displacement value for (G+8) building frame

Storey	Displacement for RC3 (mm)	Displacement for RC4 (mm)	Displacement for CF3 (mm)	Displacement for CF4 (mm)	Displacement for SE3 (mm)	Displacement for SE4 (mm)
Storey9	23.329	24.878	22.254	23.703	17.883	18.99
Storey8	22.26	23.844	21.24	22.72	17.14	18.276
Storey7	20.704	22.358	19.751	21.291	15.987	17.174
Storey6	18.615	20.368	17.744	19.367	14.385	15.641
Storey5	16.059	17.934	15.273	17	12.373	13.712
Storey4	13.092	15.107	12.396	14.241	10.002	11.432
Storey3	9.772	11.933	9.176	11.137	7.34	8.849
Storey2	6.182	8.458	5.709	7.743	4.495	6.031
Storey1	2.517	4.636	2.242	4.073	1.724	3.063

V. CONCLUSIONS

The displacement for building having RCC column showing larger displacement value as compared to the composite columns. It has been seen that RC1 (RCC column) is 26.35% more than the SE2 (steel encased column). As, the building frame height increases from (G+3) to (G+8) then there is increase in displacement by 32% which shows that increase in height of building led to increase in displacement irrespective of column material.

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