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## Prediction of Coefficient of Discharge for Oblique Sharp Crested Weir using ANN Model

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Abstract: The application of artificial neural networks for solving complex civil engineering problems is of huge importance. This paper presents some of the positive aspects of the neural network's model that are used for the determination of the coefficient of discharge of oblique sharp crested weir. Sharp crested weirs are used to measure flow rate and control upstream water surface in irrigation canals and laboratory flumes. The main advantages of such weirs are ease of construction and capability of measuring a wide range of flows with sufficient accuracy. ANN models have been developed to predict the discharge coefficients of oblique sharp-crested weirs for free flow cases using Borgheiet al.'s experimental data. Keywords: Oblique sharp crested weir; ANN; Discharge coefficient; Discharge measurement.

## I. INTRODUCTION

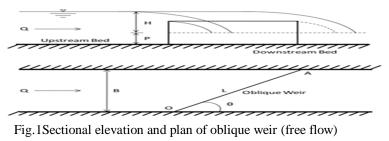
Especially in the field of irrigation and drainage engineering, sharp-crested weirs are widely used for measuring flow. In case of limited channel width when higher discharge is required at a relatively lower head use of an oblique weir is advantageous over the normal weir. Oblique alignment of the weir increases the effective weir length beyond the channel width which consequently increases the efficiency of the weir. A weir is basically an obstruction in an open channel flow path. When the water level downstream of the weir is above the crest level of the weir, then the weir is said to be submerged weir, the accuracy of measurement should not be expected at this point.

A range of measurement techniques was developed by Boss (1989) and USBR (1997). Thin-plate weirs are commonly used as measuring devices enabling an accurate discharge measurement with simple instruments. The commonly used cross sections of sharp-crested weirs are rectangular, trapezoidal, and triangular. Because of the large loss of accuracy, designing thin-plate weirs for submergence should be deliberately avoided. However, submergence may happen unexpectedly or may be temporarily necessary.

Rectangular sharp-crested weirs are of three types: (a) fully contracted, (b) partially contracted and (c) full width. If the channel width is restricted, different types of weir change from type (a) to (c). With the limitation of channel width, when the amount of discharge increases, the head of water upstream of the weir increases. However, there is a disadvantage to this kind of weir, because the higher water head means higher channel walls which are more expensive.

Borghei et al. (2003) conducted an experiment in a rectangular concrete channel of length 6600mm, width 520mm, and height 800mm. The bed slope of the channel was taken as 0.005. A standard sharp-crested weir made up of plexiglass of different heights and lengths were used. Discharge was kept between the range  $0.00845m^3$ /s, and  $0.037 m^3$ /s. The experimental data set comprises a total number of 95 runs for free flow. Experiments for free flow cases were conducted on weirs of different weir lengths (520, 595, 672, 728, 801, 876, 1030,1175mm) in eight sets. Each set corresponds to a weir height (511, 505, 506, 503, 505, 460, mm) have been used to conduct experiments in seven sets. Oblique weir is the most general case in which the weir is aligned at an angle of  $\theta$  with respect to the channel axis (Fig.1).

Ayaz and Mansoor (2018) developed ANN model to predict the discharge coefficient of oblique sharp-crested rectangular weir by using Borghei et al. (2003) experimental data. In the present study, ANN models have been developed to predict the discharge coefficient of oblique sharp-crested weir for free flow.





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## TABLE I.

Ranges of geometric and flow variables (Borghei et al., 2003).

Variables	Unit	Values (Free flow)
Oblique angle $(\theta)$	0	0-64
Discharge (Q)	$m^3/s$	0.008-0.037
Weir length (L)	mm	520-1175
Channel width (B)	mm	520
Weir height (P)	mm	460-511
Water head (H)	mm	40-106
Water head (H)	mm	-
Number of runs	-	95

### II. THE FORMULA FOR FLOW MEASUREMENT

The general discharge formula for flow measurement for weirs is:

$$Q = CH^n$$

where Q is discharge, H is the head of water above the weir crest, n is a constant that depends on the weir geometric type and C is a variable depending on the weir type and flow characteristics.

For a general rectangular sharp-crested weir, n is equal to 1.5 and C is expressed as

$$C = \frac{2}{3}C_D\sqrt{2gL}$$

where  $C_d$  is the discharge coefficient, L is the effective weir length and g is the gravitational acceleration.

$$c_d = a + b \frac{H}{P}$$

in which P is the weir height, and a and b are numerical coefficients to be found experimentally.

The standard Formula for Rectangular oblique sharp- created weir for calculating discharge coefficient is given by Borghei et al. (2003).

$$Cd = 0.0611 + (0.075 - 0.611\beta) \frac{H}{p} - 0.075\beta \left(\frac{H}{P}\right)^2$$

## III. ARTIFICIAL NEURAL NETWORKS (ANN) – BASIC CONCEPT

An artificial neural network (ANN) is an information-processing paradigm that is inspired by the way biological nervous systems, such as the brain, process information. It is composed of a large number of highly interconnected processing elements (neurons) working in unison to solve specific problems. Marijana Lazarevskai give detail application on ANN in civil engineering. Neurons are arranged in layers, including an input layer, hidden layers, and an output layer. There is no specific rule that dictates the number of hidden layers. The function is established largely based on the connections between the elements of the network. In the input layer, each neuron is designated for one of the input parameters. A detail description on neural computing and basic concepts of ANN is available in Zurada (1990) and Schalk off (1997).

The network learns by applying the back-propagation algorithm, which compares the neural network simulated values with the actual values and calculates the estimation errors. (Fig.2) Shows the data set in the network is divided into a learning data set, which is used to train the network, and a validation data set, which is used to test the network performance. In the present study, the neural network fitting tool of MATLAB 7.5 was used.

After training the network, verification is conducted until the success of the training can be established.

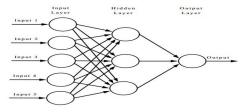


Fig.2 Model of artificial neuron



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The artificial neuron receives the input signals and generates the output signals. Every data from the surrounding or an output from other neurons can be used as an input signal. The model for an artificial neuron is shown in (Fig. 3).

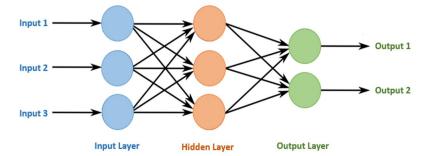


Fig. 3 Model of one layered artificial neural network

## IV. DEVELOPMENT OF ANN MODEL

Weir length (L), Weir height (P), and Water Head (H) are the three main factors that affect the Coefficient of Discharge (C<sub>d</sub>) of a oblique sharp-crested weir. The primary goal of this research is to develop of ANN models using three different networks Levenberg Marquardt, Bayesian Regularization, Scaled Conjugate Gradient to predict the Coefficient of discharge (C<sub>d</sub>) for an oblique sharp-crested weir. A detailed description about the training of neural networks using Levenberg–Marquardt algorithm is available in Hagan et al. (1996) and Hagan and Menhaj (1994).Every model consists of three layers input layer, hidden layer, and output layer. There are three neurons in the input layer and one neuron in the output layer. Fitting tool further randomly divided the experimental data as 70% for training 15% for validation and 15% for testing. Water head (H), weir height (P), and weir length (L) have been used as inputs corresponding to three neurons of input layer while the discharge coefficient (C<sub>d</sub>) as output to the ANN models. A flow chart explaining execution of work is shown below in (Fig. 5). The performance results of all three models are shown in Table 2,3 and 4.

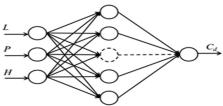


Fig. 4 ANN model (free flow)

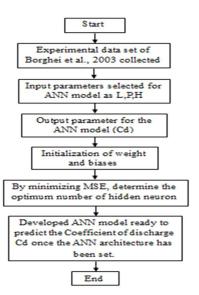


Figure. 5 Flow chart for ANN model for prediction of coefficient of discharge,



			Coefficie en C <sub>d</sub> &		isch	arge l	oy AN	N m	odel d	levelope	d by Lav	venl	berg N	Marqu	ardt N	ethod a	gorithm an
	-	L=520m B=520m P=511m	m					B=5	595mm 520mm 505mm	_					L= 67 B= 52 P= 50	0mm	
H:mm	Q:m²/s	Cd	Cdon	% Error	] [	H:mm	Q:m	<sup>3</sup> /s	C <sub>d</sub>	Cdow	% Error	Γ	H:mm	Q:m <sup>3</sup> /	s C <sub>d</sub>	Cdow	% Error
106.2	0.033	0.634	0.634	0.000	11	100.3	0.03	5	0.645	0.644	-0.155	- 5	94.7	0.036	0.625	0.622	-0.482
100.2	0.030	0.632	0.631	0.00	1 F	93.9	0.03	_	0.644	0.645	-0.311		91.0	0.030	0.623	0.627	0.637
95.1	0.028	0.631	0.629	-0.32	11	90.6	0.03	50	0.640	0.640	0	1	88.1	0.032	0.622	0.622	0
84.7	0.023	0.629	0.628	-0.16	1 C	87.1	0.02	28	0.638	0.637	0	1	82.9	0.029	0.620	0.618	-0.323
78.3	0.021	0.627	0.627	0.00	1 C	82.3	0.02	26	0.636	0.635	-0.157		74.6	0.025	0.620	0.618	-0.323
72.2	0.018	0.626	0.626	0.00	1 C	76.1	0.02	23	0.634	0.632	-0.157		71.0	0.023	0.621	0.617	-0.648
68.3	0.017	0.625	0.625	0.00	1 L	70.4	0.02	20	0.630	0.629	-0.158	(	67.8	0.021	0.618	0.617	-0.162
62.4	0.014	0.625	0.623	-0.32	1[	65.6	0.01	8	0.629	0.627	-0.159	(	62.4	0.019	0.617	0.617	0
57.4	0.01	0.622	0.622	0.000	1[	61.6	0.01	6	0.627	0.625	-0.159	-	59.3	0.017	0.616	0.616	0
50.6	0.010	0.621	0.620	-0.11	1[	57.5	0.01	5	0.626	0.623	-0.481	-	50.3	0.013	0.615	0.614	0
47.7	0.009	0.621	0.619	-0.21	1 [	49.7	0.01	2	0.621	0.620	0	4	45.8	0.011	0.614	0.613	-0.163
44.0	0.009	0.621	0.619	-0.32	4 [	42.8	0.00	19	0.618	0.619	0.165	4	41.3	0.010	0.613	0.612	-0.163
		L=728mr B=520mr P=506mr	n		_ ,				8–520mn P–503mn						L=876 B=520 P=505	mm	
H:mm	Q:m <sup>3</sup> /s	C <sub>d</sub>	Caun	% Error		H:mm	Q:n	2 <sup>3</sup> /s	$C_d$	Caun	% Error		H:mm	Q:m <sup>3</sup>	S Ca	Caun	% Error
91.2	0.035	0.608	0.612	0.653	11	89.1	0.0	36	0.580	0.58	0	1	95.0	0.036	0.56	0.568	-0.176
85.2	0.033	0.609	0.608	-0.164	11	82.4	0.0		0.586	0.586	0	1	82.2	0.035		-	-1.225
79.2	0.029	0.610	0.607	-0.494	11	77.2	0.0	29	0.588	0.588	0	1	79.7	0.033	0.57	7 0.574	-0.522
75.2	0.026	0.608	0.607	-0.164	] [	72.6	0.0	27	0.591	0.586	0.853		76.4	0.031	0.57	0.577	0
71.1	0.024	0.609	0.607	-0.329		67.7	0.0		0.593	0.593	0	ΙL	70.2	0.027		_	0
67.6	0.023	0.609	0.607	-0.164	┤┟	64.9	0.0	_	0.595	0.594	-0.168	┥┝	66.0	0.025	_	_	0.171
63.7	0.021	0.609	0.608	0	┥┟	61.9 56.5	0.0		0.597	0.595 0.598	-0.336	┥┝	59.8 58.4	0.022	0.58	_	-0.339
57.1 54.2	0.017	0.610	0.609	0	┥┟	55.3	0.0		0.598	0.598	0	┥┝	54.3	0.021			-0.339
50.1	0.010	0.610	0.609	0	-	50.0	0.0		0.602	0.601	-0.166	+	53.1	0.018	_	_	0
46.0	0.012	0.610	0.608	0	┥┟	46.7	0.0		0.603	0.603	0	+	46.1	0.015		-	0
40.9	0.010	0.610	0.608	0	11	41.4	0.0	12	0.606	0.606	0	1	40.9	0.012	0.59	0.600	0.166
				L=1030r B=520m P=505m	m		%				-	B=	1175mm 520mm 460mm	- -	a/	1	
		H:mm	Q:m <sup>3</sup> /s	Cd	C.	··· E	rror			H:mm	Q:m <sup>3</sup> /s	0		Cian	% Error		
		77.3	0.036	0.555	0.5	54 -0	.180			69.6	0.034	0.5	542 (	0.542	0	]	
		75.6	0.035	0.558	0.5	58	0			68.0	0.033	0.5	544 (	).544	0	]	
		74.4	0.034	0.561	0.5	60 -0	.178			65.7	0.031	0.5	547 (	0.547	0		
		72.0	33.24	0.566	0.5	64 -0	.354			62.8	0.030	0.5	552 (	0.552	0	]	
		67.8	0.030	0.569	0.5	70 0	.175			60.7	0.028	0.5	554 (	).555	0.180		
		64.9	0.028	0.573	0.5	73	0			57.4	0.026	0.5	559 (	0.559	0	]	
		61.1	0.026	0.576	0.5	76	0			55.1	0.025	0.5	562 (	0.567	0.881	]	
		57.2	0.024	0.582	0.5	79 -0	.518			52.6	0.023	0.5	566 (	0.566	0	]	
		52.5	0.021	0.583	0.5	83	0			50.3	0.022	0.5	571 (	0.570	-0.175	]	
		50.4	0.020	0.586	0.5	85 -0	.170			47.1	0.020	0.5	575 (	).575	0	1	
		46.8	0.018	0.588	0.5	88	0			42.9	0.017	0.5	582 (	0.581	-0.175	1	
				0.594	0.5												



## Table 3. Prediction of Coefficient of discharge by ANN model developed by Bayesian Regularization Method algorithm and percentage error between Cd& Cdann:

L=595mm

B=520mm

L	-520mm
B	-520mm
$P_{\pi}$	-511mm

H:mm	Q:m <sup>3</sup> /s	C <sub>d</sub>	Cian	% Error
106.2	0.033	0.634	0.640	0.94
100.2	0.030	0.632	0.629	-0.48
95.1	0.028	0.631	0.624	-1.12
84.7	0.023	0.629	0.622	-1.13
78.3	0.021	0.627	0.628	0.16
72.2	0.018	0.626	0.635	1.42
68.3	0.017	0.625	0.640	2.34
62.4	0.014	0.625	0.644	2.95
57.4	0.01	0.622	0.642	3.12
50.6	0.010	0.621	0.628	1.11
47.7	0.009	0.621	0.618	-1.80
44.0	0.008	0.621	0.600	-0.98

	P=505mm								
H:mm	Q:m <sup>3</sup> /2	Cd	Cim	% Error					
100.3	0.035	0.645	0.648	0.462					
93.9	0.032	0.644	0.642	-0.311					
90.6	0.030	0.640	0.641	0.156					
87.1	0.028	0.638	0.641	0.468					
82.3	0.026	0.636	0.644	1.242					
76.1	0.023	0.634	0.653	2.909					
70.4	0.020	0.630	0.664	5.120					
65.6	0.018	0.629	0.673	6.537					
61.6	0.016	0.627	0.678	7.522					
57.5	0.015	0.626	0.680	7.941					
49.7	0.012	0.621	0.668	7.035					
42.8	0.009	0.618	0.638	3.134					

L= 672 mm B= 520mm P=506mm

H:mm	Q:m <sup>3</sup> /s	C <sub>d</sub>	Cia	% Error
94.7	0.036	0.625	0.623	-0.321
91.0	0.033	0.623	0.618	-0.809
88.1	0.032	0.622	0.615	-1.138
82.9	0.029	0.620	0.617	-0.486
74.6	0.025	0.620	0.614	-0.977
71.0	0.023	0.621	0.617	-0.648
67.8	0.021	0.618	0.621	0.483
62.4	0.019	0.617	0.629	1.907
59.3	0.017	0.616	0.632	2.531
50.3	0.013	0.615	0.623	1.284
45.8	0.011	0.614	0.622	1.286
41.3	0.010	0.613	0.604	-1.490

$L^{*}$	= 728mm
B	-520mm
$P_{i}$	= 506mm

#### L= 801mm B=520mm P= 503mm

#### L= 876 mm B=520mm P= 505mm

% H:mm Q:m3/s  $C_{4}$ Cim Error 91.2 0.608 0.609 0.164 0.035 85.2 0.032 0.609 0.602 -1.162 0.029 -1.836 79.2 0.610 0.599 75.2 0.026 0.608 0.598 -1.672 71.1 0.024 0.609 0.600 -1.5 67.6 0.023 0.609 0.603 -0.995 63.7 0.021 -0.329 0.609 0.607 57.1 0.017 0.610 0.614 0.651 54.2 0.016 0.610 0.974 0.616 50.1 0.014 0.610 0.615 0.813 46.0 0.012 0.610 0.609 -0.164 40.9 0.010 0.610 0.594 2.693

H:mm	Q:m <sup>3</sup> /s	C <sub>d</sub>	Cian	% Error
89.1	0.036	0.580	0.586	1.023
82.4	0.032	0.586	0.584	-0.342
77.2	0.029	0.588	0.584	-0.684
72.6	0.027	0.591	0.587	-0.681
67.7	0.024	0.593	0.595	0.336
64.9	0.023	0.595	0.596	0.167
61.9	0.021	0.597	0.601	0.665
56.5	0.018	0.598	0.609	1.806
55.3	0.018	0.599	0.611	1.963
50.0	0.015	0.602	0.615	2.113
46.7	0.014	0.603	0.614	1.791
41.4	0.012	0.606	0.604	-0.331

H:mm	Q:m <sup>3</sup> /s	C <sub>d</sub>	Cian	% Error
95.0	0.036	0.569	0.579	1.727
82.2	0.035	0.578	0.578	0
79.7	0.033	0.577	0.578	0.173
76.4	0.031	0.577	0.578	0.173
70.2	0.027	0.581	0.579	-0.345
66.0	0.025	0.583	0.581	-0.344
59.8	0.022	0.587	0.585	-0.341
58.4	0.021	0.591	0.587	-0.681
54.3	0.019	0.592	0.590	-0.338
53.1	0.018	0.593	0.591	-0.338
46.1	0.015	0.598	0.593	-0.843
40.9	0.012	0.599	0.588	-1.870

#### L =1030mm B= 520mm P= 505mm

H:mm	Q:m <sup>3</sup> /s	C <sub>d</sub>	Cim	% Error
77.3	0.036	0.555	0.557	0.359
75.6	0.035	0.558	0.559	-5.187
74.4	0.034	0.561	0.560	-0.178
72.0	33.24	0.566	0.562	-0.711
67.8	0.030	0.569	0.567	-0.352
64.9	0.028	0.573	0.570	-0.526
61.1	0.026	0.576	0.574	-0.348
57.2	0.024	0.582	0.578	-0.692
52.5	0.021	0.583	0.583	0
50.4	0.020	0.586	0.585	-0.170
46.8	0.018	0.588	0.588	0
41.1	0.015	0.594	0.588	-1.020



H:mm	Q:m <sup>3</sup> /s	C <sub>d</sub>	Cia	% Error
69.6	0.034	0.542	0.531	-2.071
68.0	0.033	0.544	0.537	-1.303
65.7	0.031	0.547	0.540	-1.296
62.8	0.030	0.552	0.549	-0.546
60.7	0.028	0.554	0.556	0.359
57.4	0.026	0.559	0.566	1.236
55.1	0.025	0.562	0.571	1.576
52.6	0.023	0.566	0.576	1.736
50.3	0.022	0.571	0.577	1.039
47.1	0.020	0.575	0.573	-0.349
42.9	0.017	0.582	0.582	-4.488
-	-	-	-	-



			I	L=520m 8=520m P=511m	m					В	= 595mm =520mm = 505mm						B= 5	572mm 520mm 506mm	_	
I:m m	Q:m	<b>n</b> <sup>2</sup> /	C <sub>d</sub>		Cian	% Em		[	H:mm	Q:m <sup>3</sup> /s	C <sub>d</sub>	C.	~	% Error	] [	H:mm	<i>Q:m</i> <sup>3</sup> /s	C <sub>d</sub>	Cian	% Error
06.2	0.0	03	0.63	4	0.646	1.86		Ì	100.3	0.035	0.645	0.637	7 1.	883	11	94.7	0.036	0.625	0.624	-0.160
00.2	0.0	03	0.63		0.645	2.02		[	93.9	0.032	0.644	0.636		044		91.0	0.033	0.623	0.623	0
5.1	0.0	-	0.63		0.645	2.17			90.6	0.030	0.640	0.635		047		88.1	0.032	0.622	0.623	0.160
4.7	0.0	-	0.62		0.645	2.48	_		87.1 82.3	0.028	0.638	0.637		668 053	┥╽	82.9	0.029	0.620	0.644 0.623	3.726
2.2	0.0	-	0.62		0.643	2.64	_		76.1	0.026	0.636	0.633		053 584	┥╽	74.6	0.025	0.620	0.623	0.481
8.3	0.0	-	0.62		0.642	2.64	_		70.4	0.023	0.630	0.631		748	┥╽	67.8	0.023	0.618	0.623	0.321
2.4	0.0	-	0.62		0.639	2.65	-		65.6	0.020	0.629	0.627		594	┥╽	62.4	0.021	0.617	0.623	0.963
7.4	0.0	-	0.62		0.635	2.19	$\neg$	ł	61.6	0.018	0.627	0.625		44	┥┟	59.3	0.019	0.616	0.623	1.123
0.6	0.0	-	0.62	21	0.628	1.11	-	ł	57.5	0.015	0.626	0.621		966	┥┟	50.3	0.013	0.615	0.621	0.966
7.7	0.0	-	0.62	21	0.624	0.48	-	ł	49.7	0.012	0.621	0.613	3 -0	.163	┨╏	45.8	0.011	0.614	0.620	0.967
4.0	0.0	08	0.62	21	0.619	-0.32	2	ł	42.8	0.009	0.618	0.602	2 -1	.827	11	41.3	0.010	0.613	0.617	0.648
				L= 728m					L= 801 mm						L= 876mm					
				B=520m ≥ 506n					<i>B</i> =520 mm <i>P</i> = 503 mm						B= 520mm P= 505mm					
			_	- 500						P	- 303 m							JUJIIII	_	
H:m	m	Q: 1	n <sup>3</sup> /s	C <sub>d</sub>	Cian		% rror		H:mm	Q:m <sup>3</sup> /s	C <sub>d</sub>	Cie		% ror		H:mm	Q:m <sup>3</sup> /s	C <sub>d</sub>	Cim	% Error
91.2		0.0	)35	0.608	0.613	0.8	15		89.1	0.036	0.580	0.59	3 2.1	92		95.0	0.036	0.569	0.576	1.215
85.2		0.0	032	0.609	0.613	0.6	52		82.4	0.032	0.586	0.59	2 1.0	013		82.2	0.035	0.578	0.576	-0.347
79.2			)29	0.610	0.613	0.4			77.2	0.029	0.588	0.59	_	575		79.7	0.033	0.577	0.578	0.173
75.2	-		026	0.608	0.614	0.5			72.6	0.027	0.591	0.59	_	68		76.4	0.031	0.577	0.580	0.517
71.1 67.6	$\rightarrow$		024	0.609	0.615	0.9			67.7	0.024	0.593	0.59	_	570 337		70.2	0.027	0.581	0.584	0.513
63.7	+	0.0	_	0.609	0.617	1.1			64.9 61.9	0.023	0.595	0.59	_	505		66.0 59.8	0.025	0.583	0.588 0.594	0.850
57.1	+		017	0.610	0.620	1.6			56.5	0.021	0.598	0.59	_	335		59.8	0.022	0.587	0.596	0.838
54.2	+		016	0.610	0.621	1.7			55.3	0.018	0.599	0.59	_	335		54.3	0.019	0.592	0.602	1.661
50.1	$\neg$		014	0.610	0.622	1.5	28		50.0	0.015	0.602	0.59	_	500		53.1	0.018	0.593	0.603	1.658
46.0	$\neg$	0.0	012	0.610	0.623	2.0	86		46.7	0.014	0.603	0.60	0 -0	.5		46.1	0.015	0.598	0.612	2.287
40.9		0.0	010	0.610	0.624	2.0	86		41.4	0.012	0.606	0.60	2 -0.	664		40.9	0.012	0.599	0.613	2.283
							1030 i 520m		n							1175mm 520mm				
							505 r		-							460mm	_			
				Ha	mm Q	m <sup>3</sup> /s	C <sub>d</sub>		Cian	% Error			H:mm	Q:1	n <sup>3</sup> /s	C <sub>d</sub>	Cier	% Error		
						0.036	0.555	-	0.543	-2.209	4	F	69.6		034	0.542	0.592	8.445	4	
					_	0.035	0.558	-	0.545	-2.385	-	⊢	68.0 65.7	-	033	0.544	0.585	7.008	-	
					2.0 3	3.24	0.566	5	0.550	-2.909	1	E	62.8	0.	030	0.552	0.563	1.953		
						0.030	0.569	-	0.558	-1.971	4	F	60.7	+	028	0.554	0.554	0	_	
						0.028	0.57	-	0.563	-1.776	-	⊢	57.4 55.1		026	0.559	0.540	-3.518	_	
				57.2		0.024	0.582	-	0.580	0	1	F	52.6	-	023	0.566	0.520	-8.846	-	
					_	.021	0.583	-	0.591	1.353	]		50.3		022	0.571	0.511	-11.74	_	
						0.020	0.58	-	0.596	1.677	-	F	47.1		020	0.575	0.497	-15.69	_	
					_	0.018	0.588	-	0.605	2.809	-	F	42.9	+	017	0.582	0.479	-21.50	-	



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## TABLE 4.

Percentage error between theoretical coefficient of discharge and predicted coefficient of discharge by (Levenberg Marquard algorithm).

L:mm	B:mm	P:mm	Maximum Error:%	Minimum Error:%	
520	520	511	-0.32	0.00	
595	520	506	-0.48	0	
672	520	506	0.63	0	
728	520	506	0.65	0	
801	520	503	-0.853	0	
876	520	505	-0.522	0	
1030	520	505	-0.518	0	
1175	520	460	0.881	0	

## TABLE 5.

Percentage error between theoretical coefficient of discharge and predicted coefficient of discharge by (Bayesian Regularization Method).

L:mm	B:mm	P:mm	Maximum Error: %	Minimum Error: %	
520	520	511	3.12	0.16	
595	520	506	7.94	0.15	
672	520	506	1.90	-0.32	
728	520	506	-2.69	0.162	
801	520	503	2.11	0.336	
876	520	505	-1.87	0	
1030	520	505	-5.187	0	
1175	520	460	-4.488	0.359	

## TABLE 6.

Percentage error between theoretical coefficient of discharge and predicted coefficient of discharge by (Scaled Conjugate Method).

L:mm	B:mm	P:mm	Maximum Error: %	Minimum Error: %	
520	520	511	2.65	-0.32	
595	520	506	2.66	-0.16	
672	520	506	3.72	0	
728	520	506	2.08	0.48	
801	520	503	2.19	-0.5	
876	520	505	2.287	-0.34	
1030	520	505	3.727	0	
1175	520	460	-21.503	0	



## COMPARISON BETWEEN CD AND CDANN PREDICTED BY LAVENBERG MARQUARDT METHOD

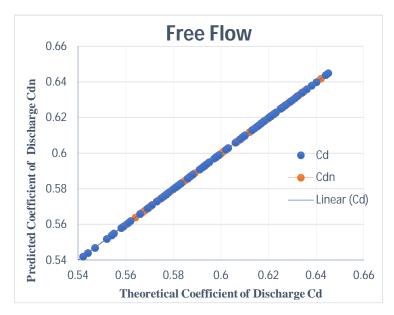


Fig 5. Graph between theoretical coefficient of discharge and predicted coefficient of discharge.

## VI. DISCUSSION ON PERFORMANCE RESULTS

Using experimental data of free flow, we developed ANN model's by using three algorithms available in matlab, Levenberg Marquardt, Bayesian Regularization, Scaled Conjugate Gradient.

ANN model developed by above three algorithm, model developed by Levenberg Marquardt algorithm is the best to predict the  $Cd_{ann}$ , as seen in following table percentage error of Levenberg Marquardt algorithm is between  $\pm 0.881\%$ .

Method's	% Error
Levenberg Marquardt	0.881 to 0
Bayesian Regularization	7.94 to 0.15
Scaled Conjugate Gradient	-21.50 to 0

## VII. CONCLUSIONS

- 1) The Levenberg Marquardt, Bayesian Regularization, Scaled Conjugate gradient algorithms are used in this study to develop the ANN models to predict the coefficient of discharge for oblique sharp crested weir's for free flow using experimental data of Borghei etal.(2003). The percentage error between the therotical coefficient of discharge and predicted coefficient of discharge is between ±0.881%.
- It is concluded that the ANN model developed by Levenberg Marquardt method is accurate and recommended for prediction of coefficient of discharge as percentage error between therotical coefficient and predicted coefficient of discharge is in the range of ±0.881%.
- 3) Modern multidisciplinary fields like artificial neural networks are a good illustration of how to solve a variety of engineering challenges that conventional modelling and statistical approaches were unable to address.

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## NOTATION

- $C_d \qquad \text{Theoretical coefficient of oblique weir.}$
- C<sub>dann</sub> Predicted coefficient of discharge
- H Head over weir.
- L Oblique weir length
- P Oblique weir height
- B Channel width
- Q Discharge
- $\theta$  Oblique weir angle

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